General Notes: (updated January 20, 2006 addition underlined)

1) If any two or more specified criteria conflict, the more severe governs.
2) The primary portion of this book applies only to Exposure B, in Exposure C, use tables in Supplement C.
3) The minimum extrusion wall thickness is .040" (FBC 2002.3.1). Calculations involving properties of extrusions utilize an implied minimum nominal wall thickness of .045". Thus, wherever an extrusion is specified as having a wall thickness of .045", it may be considered to be an absolute actual minimum wall thickness of .040".
4) Unless noted otherwise, extrusions depicted in this book are aluminum alloy 6063-T6 meeting ASTM B 221.
5) For the purposes of this publication, a screen enclosure (Chapter 1) is defined as a structure with screen walls and screen roof. The screen room (Chapter 2) is defined as a structure with screen walls and a solid roof, with or without removable vinyl or acrylic wind break panels.

Chapter 1: All loads were taken from Table 2002.4, Florida Building Code. All loads are for Exposure B, for heights 30 ft or less (except in Supplement C). Wall loads are the more severe Load Case A values, except for cable bracing (Table 211) which utilizes Load Case B. Screen Roof values are 10 lbs/sqft per Note 2 and Section 1606.1.2.

Chapter 2: Screen rooms must be designed by Type as differentiated by Table 200. All wall loads were taken from Table 2002.4, Florida Building Code. All loads are for Exposure B, for heights 30 ft or less (except as noted in Supplement C). Wall loads are the more severe Load Case A values, except for cable bracing (Table 211) which utilizes Load Case B. Solid Roof design pressure (wind load) determined by component and cladding values using internal pressure coefficients prescribed in ASCE7-98 and a GCp of .75 (derived from wind tunnel testing) with a [roof] Live Load of 10 lbs/sqft.

Chapter 3: Wind load on screen per Table 2002.4 (FBC), Exposure B, 30 feet high or less (except as noted in Supplement C). Load on guardrail (and posts) is 50 lbs/linear foot or 200 lb point load, whichever is greater. No compression load was applied to posts. Table 305 is furnished as a guide to using aluminum sections as support columns in conventionally framed structures, tabular values are allowable axial compression loads in pounds.

Chapter 4: Design loads were taken from Tables 1606.2A and 1606.2B (FBC) for roof slopes from 0 to 10 degrees in Zone 2 (edge strip). An occupancy importance factor of 1.0 (Table 1606) was used. Roof panel, beam span and connection design uses Component and Cladding loads (1606.2B) applied by effective wind area for span and spacing of component under consideration. Main force resisting system loads (1606.2A) were used for the foundation and shear loads.

Chapter 5: Attached covers can be either Type I or Type II, design by structure type as per Chapter Introduction and Table 500. Design loads were calculated from ASCE7-98 by type of structure for roof slopes from 0 to 10 degrees in Zone 2 (edge strip). An occupancy importance factor of 0.77 (Table 1606) was used. Roof panel, beam span and connection design uses Component and Cladding loads applied by effective wind area for span and spacing of component under consideration. Main force resisting system loads were used for the foundation loads. Live load is 20 lbs/sqft, but uplift load governs throughout.

Chapter 6: Design loads were calculated from ASCE7-98 for 'Open Buildings' for roof slopes from 0 to 10 degrees (Table 6-9). An occupancy importance factor of 0.77 (Table 1606) was used. Design loads for gabled free-standing structures were based upon wind tunnel research.

Chapters 8 and 9: An occupancy importance factor of 0.77 (Table 1606) was used.